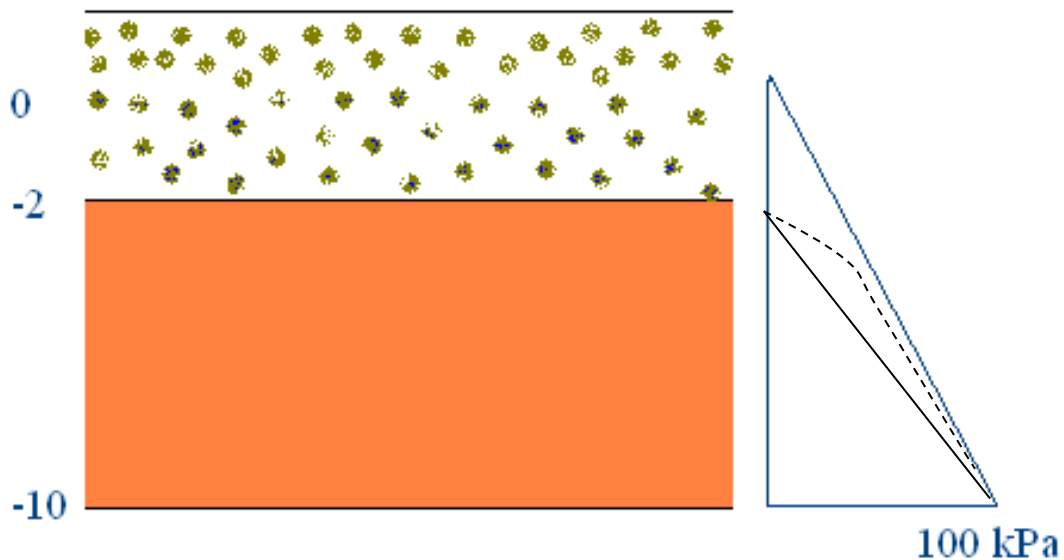
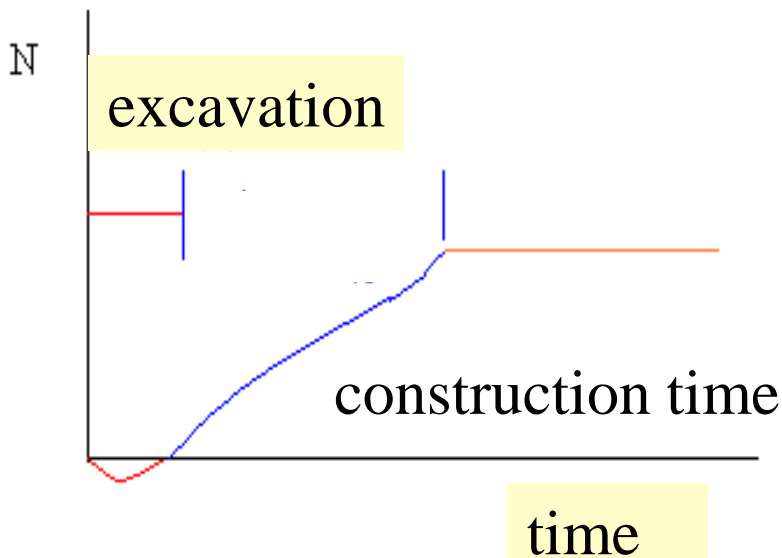


Triangular distribution of initial excess pore pressure



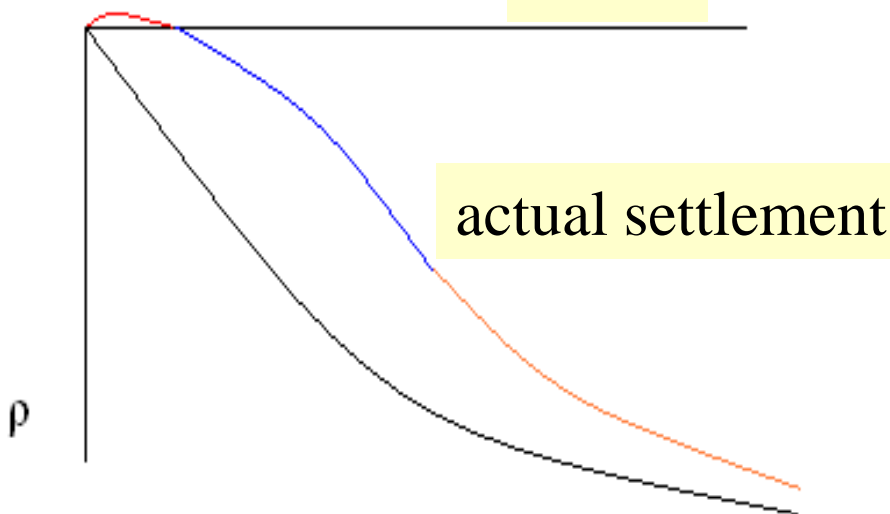
- **Excess pore pressure dissipation**
- →
- **Increase in effective stress**
- **Lowering the water table**
- **Pumping of water at the bottom of the clay layer**

Taylor (1948)



□ $t < t_{\text{construction}}$
 $\rho_t = \rho_t' * N/N_{\text{total}}$

where $t' = 0.5t$

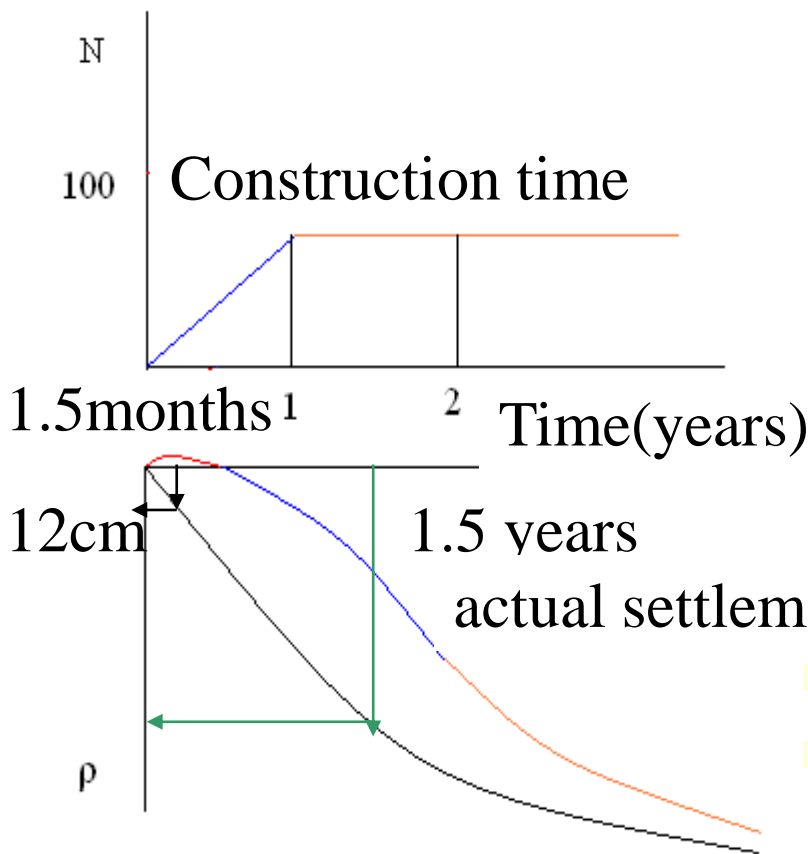


□ $t > t_{\text{construction}}$
 $\rho_t = \rho_t'$

Where

$t = t - 0.5t_{\text{constr.}}$

Linear increment of construction load



- $t < t_{\text{construction}}$
- $\rho t = \rho t' * N / N_{\text{total}}$
- where $t' = 0.5t$

- $t > t_{\text{construction}}$
- $\rho t = \rho t'$ where
- $t = t - 0.5t_{\text{const}}$

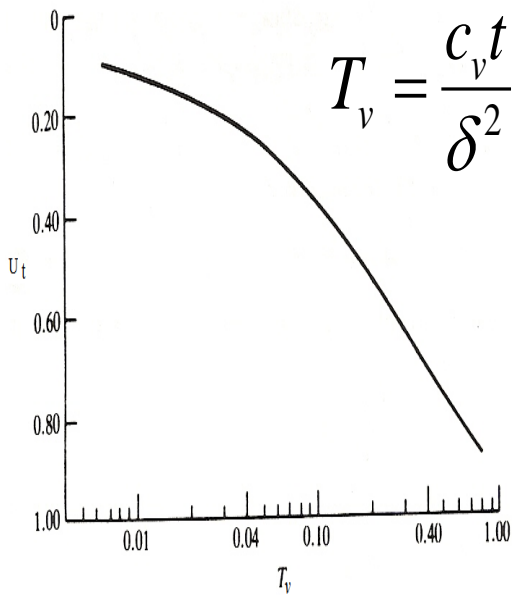
- **$t = 3$ months**
- **$\rho t = \rho t' * 0.25$**
- **$N / N_0 \lambda' = 3 / 12 = 0.25$**

Find settlement after 3 months from beginning of construction

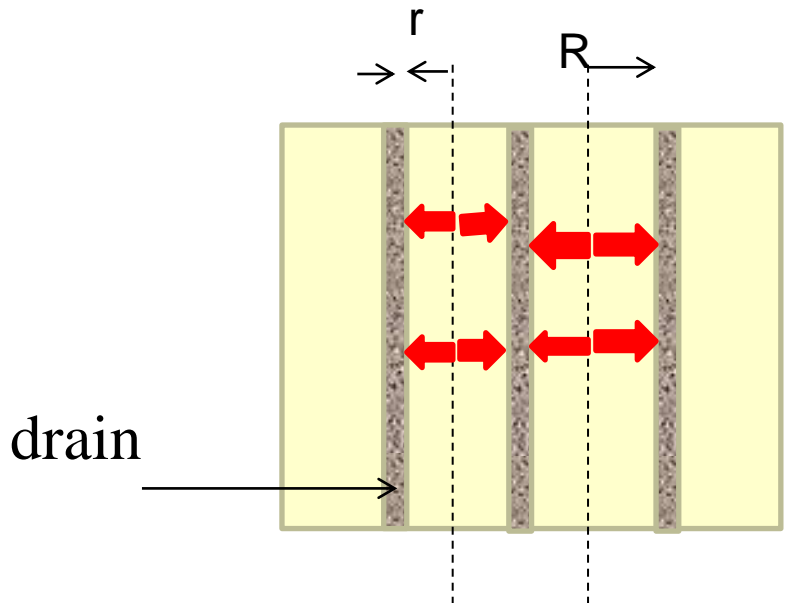
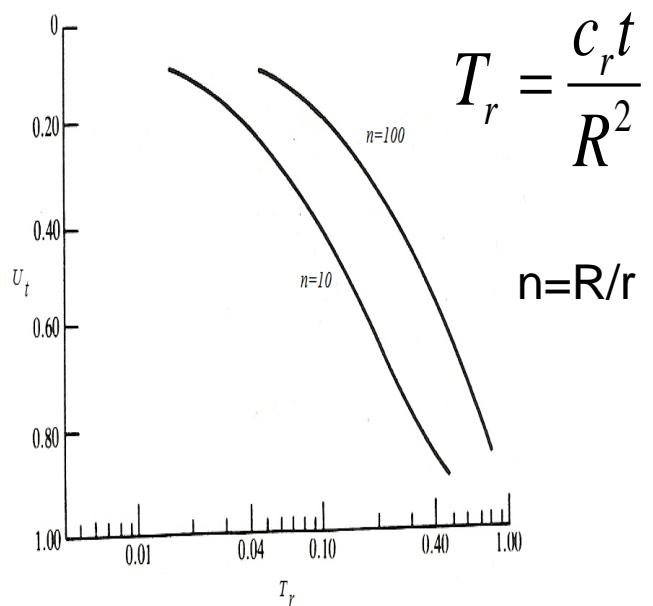
- **$t = 2$ years**
- **$\rho t = \rho t'$ where**
- **$t = t - 0.5t_{\text{const}} =$**
- **1.5 years**

One-dimensional & Radial consolidation

1 x D consolidation
(Taylor, 1948)



Radial consolidation
(Barron, 1948)



DRAINS

Anisotropic soil properties

$k_r/k_v=2-10$ for normally consolidated clays
(where r,v radial and vertical direction)

Drains

Triangular or $De=1.13 * S$

rectangular layout $De=1.05 * S$

where De : equivalent dia or diameter of influence

S : distance between drains

Define

The distance S between drains and the radius Rd of the drain for average degree of consolidation

$U>90\%$ at a time t

Calculation

The optimum design of the drains is defined by trial and error for various combinations of S & Rd

AVERAGE DEGREE OF CONSOLIDATION FOR VERTICAL & RADIAL DRAINAGE

$$1-U = (1-U_z) * (1-U_r)$$

where U_z , U_r are the average degrees of consolidation for independent vertical and radial drainage conditions

$$T_v = \frac{c_v * t}{H^2}$$

$$T_r = \frac{c_r * t}{D_e^2} \quad \text{where } D_e \text{ the drain 'diameter of influence'}$$

$$U_r = 1 - \exp(-8 * T_r / A) \quad (1)$$

&

$$A = \ln \left[\frac{R_e}{R_d} \right] - \frac{3}{4} + \left[\frac{k_r}{k_{r,s}} - 1 \right] \ln \left[\frac{R_s}{R_d} \right]$$

SMEAR ZONE ***(ΖΩΝΗ ΑΝΑΜΟΧΛΕΥΣΗΣ)***

Calculation of U_r from the exact analysis (equation 1) takes into account the existence of a smear zone having coefficient of permeability $k_{r,s}$ and diameter D_s .

assumptions:

- smear zone diameter $D_s = 2 \sim 3 * D_d$

where D_r is drain diameter

- the coefficient of permeability for the smear zone is similar to the coefficient of the soil permeability in the vertical direction

$$k_{r,s} / k_v = 1 \sim 1.5$$